JK Geotechnics GEOTECHNICAL & ENVIRONMENTAL ENGINEERS

 Θ

Field Identification Procedures (Excluding particles larger than 75 µm and basing fractions on estimated weights) Information Required for Describing Soils Laboratory Classification Criteria Group Typical Names $C_{\rm U} = \frac{D_{60}}{D_{10}} \quad \text{Greater than 4}$ $C_{\rm C} = \frac{(D_{30})^2}{D_{10} \times D_{60}} \quad \text{Between}$ of fines (fraction smaller than 75 ord soils are classified as follows: W, GF, SW, SP M, GC, SM, SC *valetilae* areas requiring use of dual symbols. Wide range in grain size and substantial amounts of all intermediate particle sizes Well graded gravels, gravel-sand mixtures, little or no fines size Clean gravels (little or no fines) Gravels More than half of coarse fraction is larger than 4 mm sieve size GW Determine percentages of gravel and and from grain s curve the secontages of finest (fraction analter than methods on percentage of finest (fraction analter than methods are table outer gratind solid to the second method of the second second second solid Last than 52, GW, GC, MA, GC, MA, GC MA, GC, MA, GC 55(to 123, doi:10.1000) name; indicate ap-percentages of sand ; maximum size; surface condition, ess of the coarse Between 1 and 3 Give typical Not meeting all gradation requirements for GR Poorly graded gravels, gravel-sand mixtures, little or no fine Predominantly one size or a range of sizes with some intermediate sizes missing GP Atterberg limits below "A" line, or PI less than 4 Above "A" I with PI betw 4 and 7 borderline ca requiring use dual symbols lit Gravels with fines (appreciable amount of Nonplastic fines (for identificati cedures see ML below) Silty gravels, poorly graded gravel-sand-silt mixtures material is sicve sizeb on pro GM a eye) identification Atterberg limits above "A" line, with PI greater than 7 soils tratification, degree of ess, cementation, conditions and naked e Plastic fines (for identification procedures see CL below) Clayey gravels, poorly graded gravel-sand-clay mixtures GC half of 1 $C_{U} = \frac{D_{40}}{D_{10}} \quad \text{Greater than 6}$ $C_{C} = \frac{(D_{20})^{2}}{D_{10} \times D_{60}} \quad \text{Between}$ Clean sands (little or no fines) Wide range in grain sizes and substantial amounts of all intermediate particle sizes Sands n half of coarse is smaller than n sieve size Well graded sands, gravelly sands, little or no fines fractions as given under field SW Coarse e than Example Between 1 and 3 mple: if/y sond, gravelly; about 20 % hard, angular gravel par-ticles 12 mm maximum size; rounded and subangularsand grains coarse to fine, about 15% non-plastic fines with low dry strength; well com-pacted and moist in place; alluvial sand; (SM) More t icle nts for SI Predominantly one size or a range of sizes with some intermediate sizes missing Poorly graded sands, gravelly sands, little or no fines Not meeting all gradation requir SP about with Atterberg limits below "A" line or Pless than 5 Atterberg limits below "A" line with Pl betweene 4 and 7 arr berderline caste requiring use of dual symbols Sands with fines (appreciable amount of More than fraction is 4 mm smallest Nonplastic fines (for identification pro cedures, see ML below) Silty sands, poorly graded sand-silt mixtures SM Plastic fines (for identification procedures see CL below) Clayey sands, poorly graded sand-clay mixtures the SC " line n sieve size is about t ifying the Fraction Smaller than 380 µm Sieve Size Dry Strength (crushing character-istics) Toughness (consistency near plastic limit) Dilatancy (reaction to shaking) 60 - Comparing soils at equal liquid limit smaller 50 Silts and clays liquid limit less than 50 Inorganic silts and very fine sands, rock flour, silty or clayey fine sands with slight plasticity Give typical name; indicate and character of pl amount and maximum coarse grains; colour xapui 40 None to slight Quick to slow None ML ity, urve ma siz soils Fine-grained soil than half of materia than 75 µm sieve s (The 75 µ wet clays of plasticity, dy clays, si size Medium to high None to very slow medium plasticity. Bravoug clays, sandy clays, ailty clays, lean clays Organic silts and organic silt-clays of low plasticity Inorganic silts, micaceous or diatomaceous fine sandy or silty soils, clastic silts of high plas-Medium CL grain s Slight to medium 10 Slow Slight OL the slips had Use CL-ML Slight to OE Slow to clays limit than Slight to MH 10 20 30 40 50 60 70 80 90 100 More Inorganic clays of high plas-ticity, fat clays Organic clays of medium to high plasticity Silts and liquid li greater t High to very high Medium to high Liquid limit ixample: Clayey silt, brown: sligh plastic; small percentage fine sand; numerous verti root holes: firm and dry place; locss; (ML) None High CH Plasticity chart None to very slow Slight to он for laboratory classification of fine grained soils med Peat and other highly organic eadily spongy identified by colour, odour, feel and frequently by fibrour PI Highly Organic Soils

UNIFIED SOIL CLASSIFICATION TABLE

 Soils possessing characteristics of two groups are designated by combinations of group symbols (eg. GW-GC, well graded gravel-sand mixture with clay fines).
Soils with liquid limits of the order of 35 to 50 may be visually classified as being of medium plasticity. Note:

 Θ



LOG COLUMN	SYMBOL	DEFINITION			
Groundwater Record		Standing water level. Time delay following completion of drilling may be shown.			
	- <u>c</u>	Extent of borehole collapse shortly after drilling.			
	-	Groundwater seepage into borehole or excavation noted during drilling or excavation.			
Samples	ES	Soil sample taken over depth indicated, for environmental analysis.			
	U50	Undisturbed 50mm diameter tube sample taken over depth indicated.			
	DB	Bulk disturbed sample taken over depth indicated.			
	DS	Small disturbed bag sample taken over depth indicated.			
	ASB	Soil sample taken over depth indicated, for asbestos screening.			
	ASS	Soil sample taken over depth indicated, for acid sulfate soil analysis.			
	SAL	Soil sample taken over depth indicated, for salinity analysis.			
Field Tests	N = 17 4, 7, 10	Standard Penetration Test (SPT) performed between depths indicated by lines. Individual figures show blows per 150mm penetration. 'R' as noted below.			
	N _c = 5 7 3R	- Solid Cone Penetration Test (SCPT) performed between depths indicated by lines. Individual figures show blows per 150mm penetration for 60 degree solid cone driven by SPT hammer. 'R' refers to apparent hammer refusal within the corresponding 150mm depth increment.			
	VNS = 25	Vane shear reading in kPa of Undrained Shear Strength.			
	PID = 100	Photoionisation detector reading in ppm (Soil sample headspace test).			
Moisture Condition	MC>PL	Moisture content estimated to be greater than plastic limit.			
(Cohesive Soils)	MC≈PL	Moisture content estimated to be approximately equal to plastic limit.			
	MC <pl< td=""><td>Moisture content estimated to be less than plastic limit.</td></pl<>	Moisture content estimated to be less than plastic limit.			
(Cohesionless Soils)	D	DRY – Runs freely through fingers.			
	M	MOIST - Does not run freely but no free water visible on soil surface.			
	W	WET – Free water visible on soil surface.			
Strength	VS	VERY SOFT - Unconfined compressive strength less than 25kPa			
(Consistency)	S	SOFT – Unconfined compressive strength 25-50kPa			
Cohesive Soils	F	FIRM – Unconfined compressive strength 50-100kPa			
	St	STIFF – Unconfined compressive strength 100-200kPa			
	VSt	VERY STIFF – Unconfined compressive strength 200-400kPa			
	н	HARD — Unconfined compressive strength greater than 400kPa			
	()	Bracketed symbol indicates estimated consistency based on tactile examination or other tests.			
Density Index/ Relative Density (Cohesionless Soils)		Density Index (I _D) Range (%) SPT 'N' Value Range (Blows/300mm)			
	VL	Very Loose <15 0-4			
	L	Loose 15-35 4-10			
	MD	Medium Dense 35-65 10-30			
	D	Dense 65-85 30-50			
	VD	Very Dense >85 >50			
	()	Bracketed symbol indicates estimated density based on ease of drilling or other tests.			
Hand Penetrometer Readings	300	Numbers indicate individual test results in kPa on representative undisturbed material unless			
	250	noted otherwise.			
Remarks	'V' bit	Hardened steel 'V' shaped bit.			
	'TC' bit				
		Tungsten carbide wing bit.			
	60	Penetration of auger string in mm under static load of rig applied by drill head hydraulics without rotation of augers.			



ROCK MATERIAL WEATHERING CLASSIFICATION

TERM	SYMBOL	DEFINITION	
Residual Soil	RS	Soil developed on extremely weathered rock; the mass structure and substance fabric are no longer evident; there is a large change in volume but the soil has not been significantly transported.	
Extremely weathered rock	xw	Rock is weathered to such an extent that it has "soil" properties, ie it either disintegrates or can be remoulded, in water.	
Distinctly weathered rock	DW	Rock strength usually changed by weathering. The rock may be highly discoloured, usually by ironstaining. Porosity may be increased by leaching, or may be decreased due to deposition of weathering products in pores.	
Slightly weathered rock	SW	Rock is slightly discoloured but shows little or no change of strength from fresh rock.	
Fresh rock	FR	Rock shows no sign of decomposition or staining.	

ROCK STRENGTH

Rock strength is defined by the Point Load Strength Index (Is 50) and refers to the strength of the rock substance in the direction normal to the bedding. The test procedure is described by the International Journal of Rock Mechanics, Mining, Science and Geomechanics. Abstract Volume 22, No 2, 1985.

TERM	SYMBOL	ls (50) MPa	FIELD GUIDE
Extremely Low:	EL		Easily remoulded by hand to a material with soil properties.
		0.03	
Very Low:	VL		May be crumbled in the hand. Sandstone is "sugary" and friable.
		0.1	
Low:	L		A piece of core 150mm long x 50mm dia. may be broken by hand and easily scored with a knife. Sharp edges of core may be friable and break during handling.
		0.3	
Medium Strength:	м		A piece of core 150mm long x 50mm dia. can be broken by hand with difficulty. Readily scored with knife.
		1	A sizes of ease 150mm long v 50mm die ease sonnet he broken hy hand ean he slightly
High:	н		A piece of core 150mm long x 50mm dia. core cannot be broken by hand, can be slightly scratched or scored with knife; rock rings under hammer.
		3	
Very High:	VH		A piece of core 150mm long x 50mm dia. may be broken with hand-held pick after more than one blow. Cannot be scratched with pen knife; rock rings under hammer.
		10	
Extremely High:	EH		A piece of core 150mm long x 50mm dia. is very difficult to break with hand-held hammer. Rings when struck with a hammer.

ABBREVIATIONS USED IN DEFECT DESCRIPTION

ABBREVIATION	DESCRIPTION	NOTES
Be	Bedding Plane Parting	Defect orientations measured relative to the normal to the long core axis
CS	Clay Seam	(ie relative to horizontal for vertical holes)
J	Joint	
P	Planar	
Un	Undulating	
S	Smooth	
R	Rough	
IS	Ironstained	
XWS	Extremely Weathered Seam	
Cr	Crushed Seam	
60t	Thickness of defect in millimetres	